Abridged version

KAWASAKI STEEL TECHNICAL REPORT No.6 (September 1982)

Ultimate Strength of "T"-shaped Connection Sheet Piles for Cellular Sheet Pilings with Straight Web Steel Sheet Piles

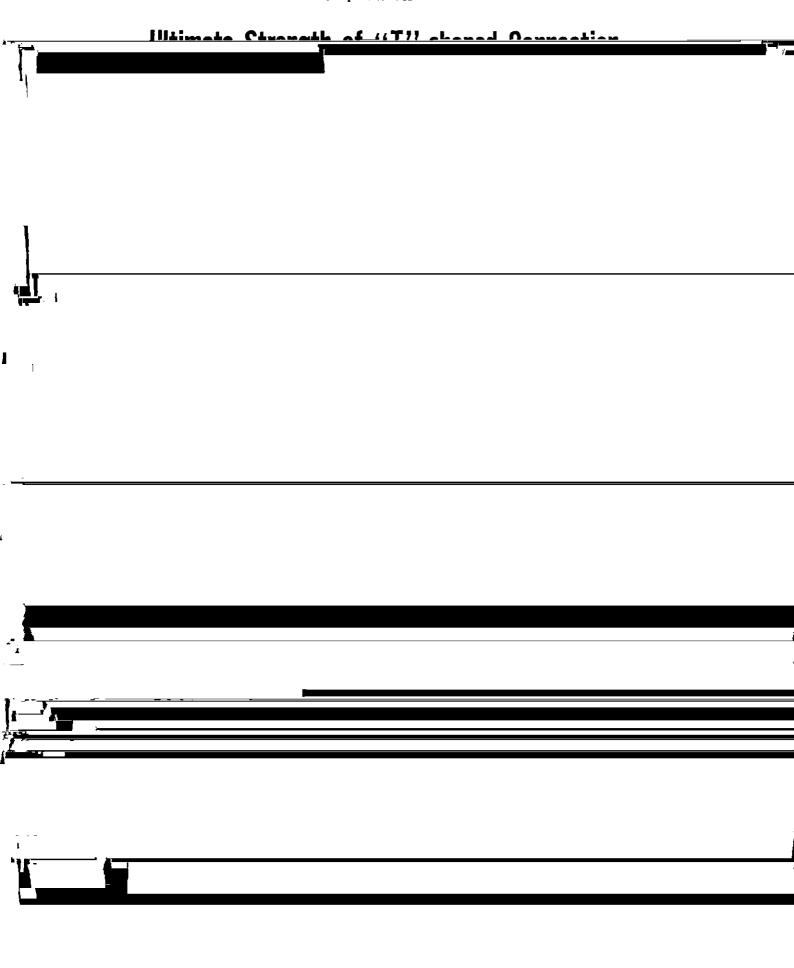
Michihiko Hara, Yutaka Kawai, Eisuke Yamanaka, Takafumi Hashimoto, Yasuhiko Ueki, Hiroo Nakagawa

Synopsis:

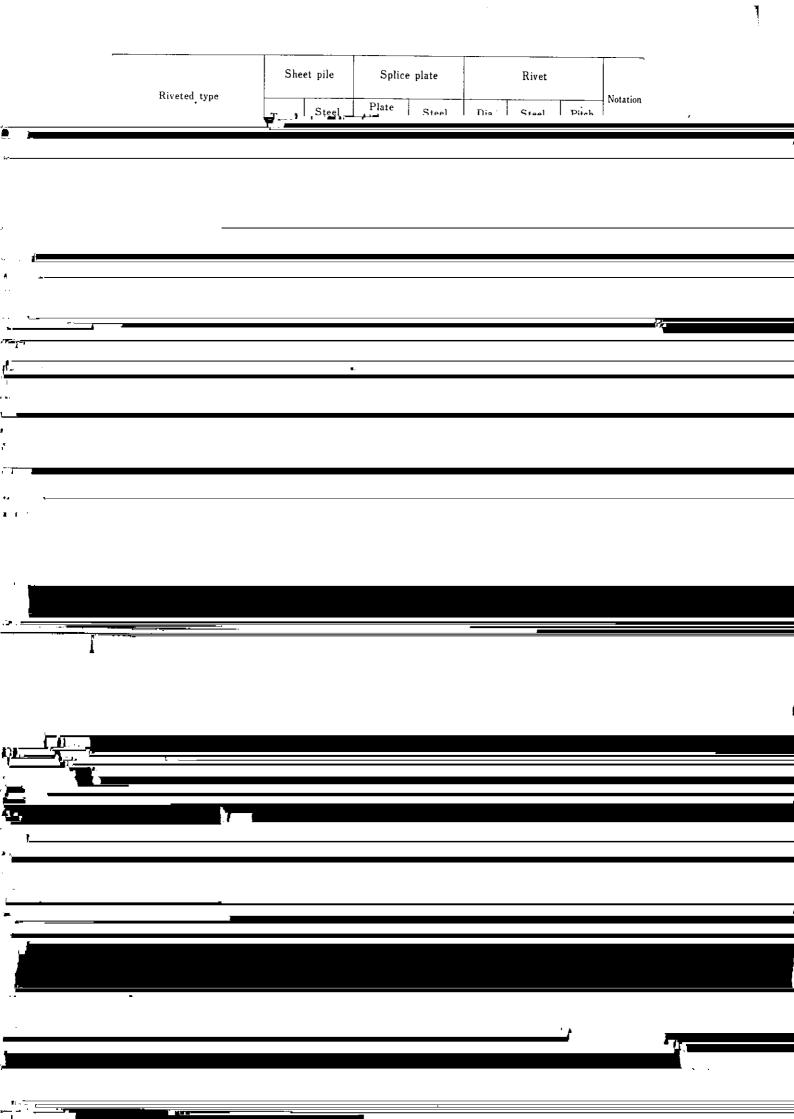
In cellular sheet piling structures, connection sheet piles, required at the intersection of walls between are cells and circular cells, are subjected to the most severe stress and deformation conditions among structural components and so they may lead to fatal failures. For the purpose of studying ultimate strength of "T" connection sheet piles, which are used for almost all types of cellular cofferdams in Japan and developing new connection sheet piles with higher strength, a series of two directional simultaneous tensile tests were conducted by using several types of about one-meter-long prototype "T" connection sheet piles. For design purposes, test results obtained were illustrated as interaction curves of ultimate strength at connections concerning hoop tensions in circular cells and are cells for every structural details, and also estimation methods for ultimate strength of typical types of connections were discussed. Additional tensile tests were also conducted on three types of "T" connection sheet piles for newly developed straight web sheet piles with higher interlock strength than that of conventional ones.

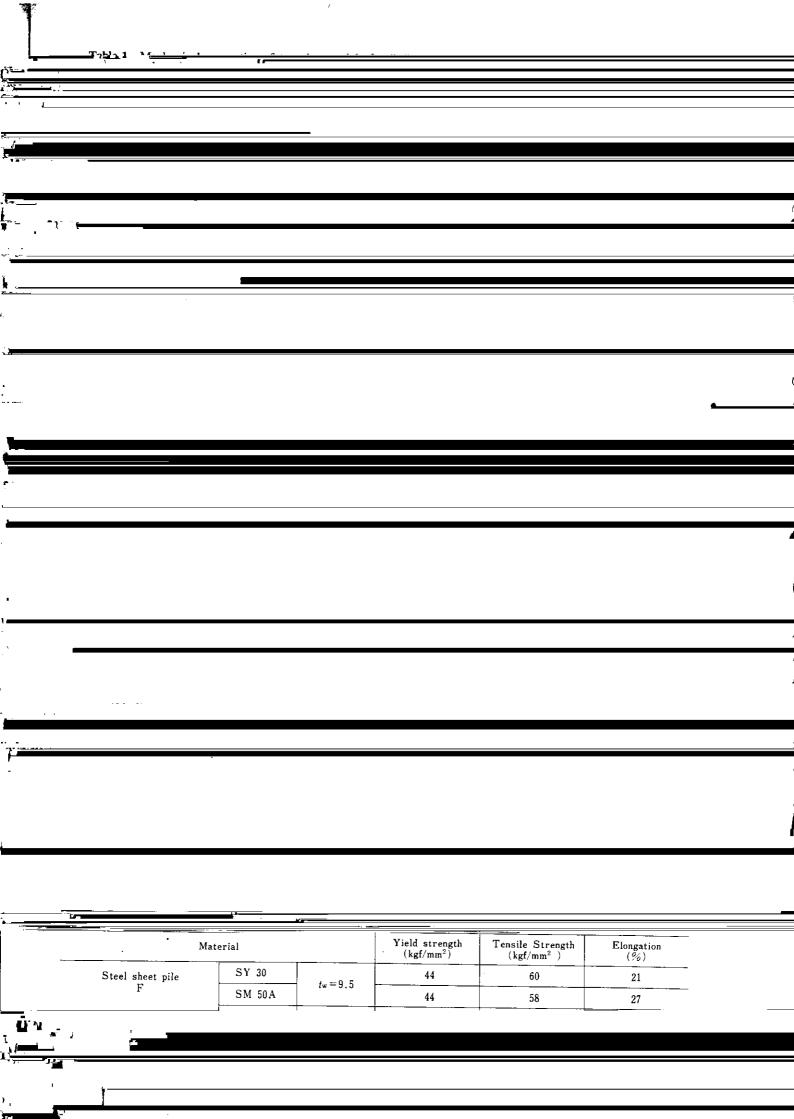
(c) JFE Steel Corporation, 2003

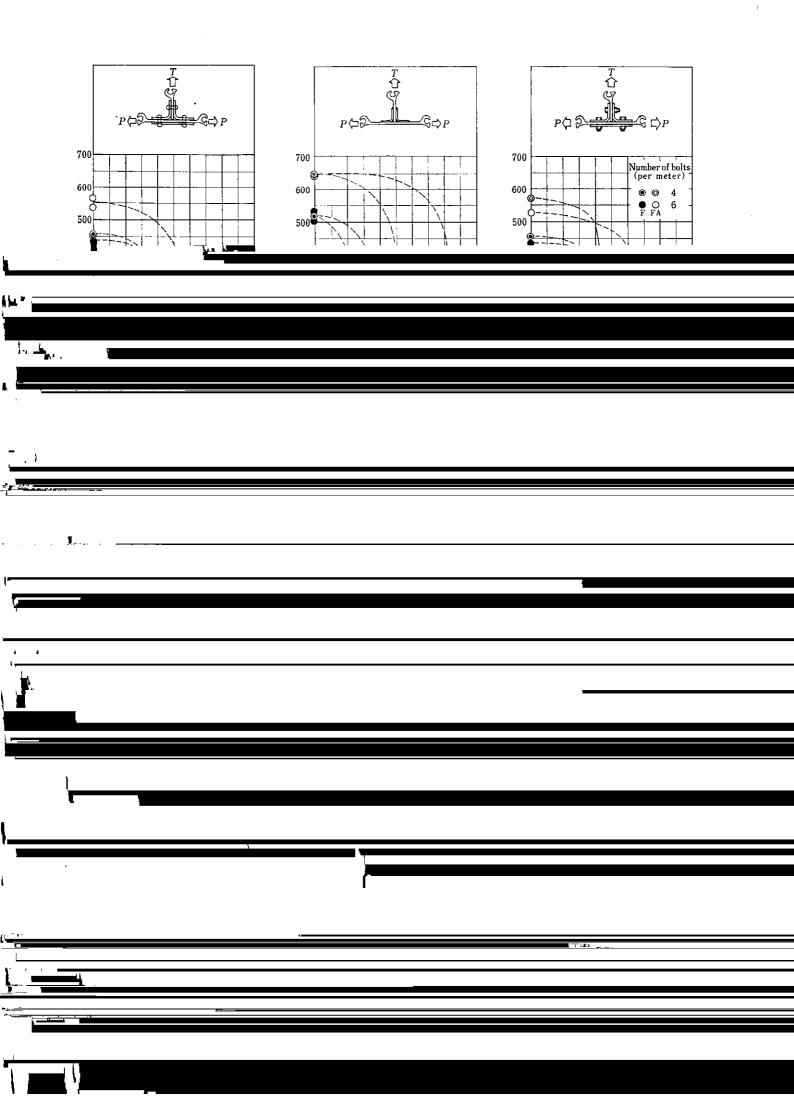
The body can be viewed from the next page.



Back filling Arc cells Circular cell







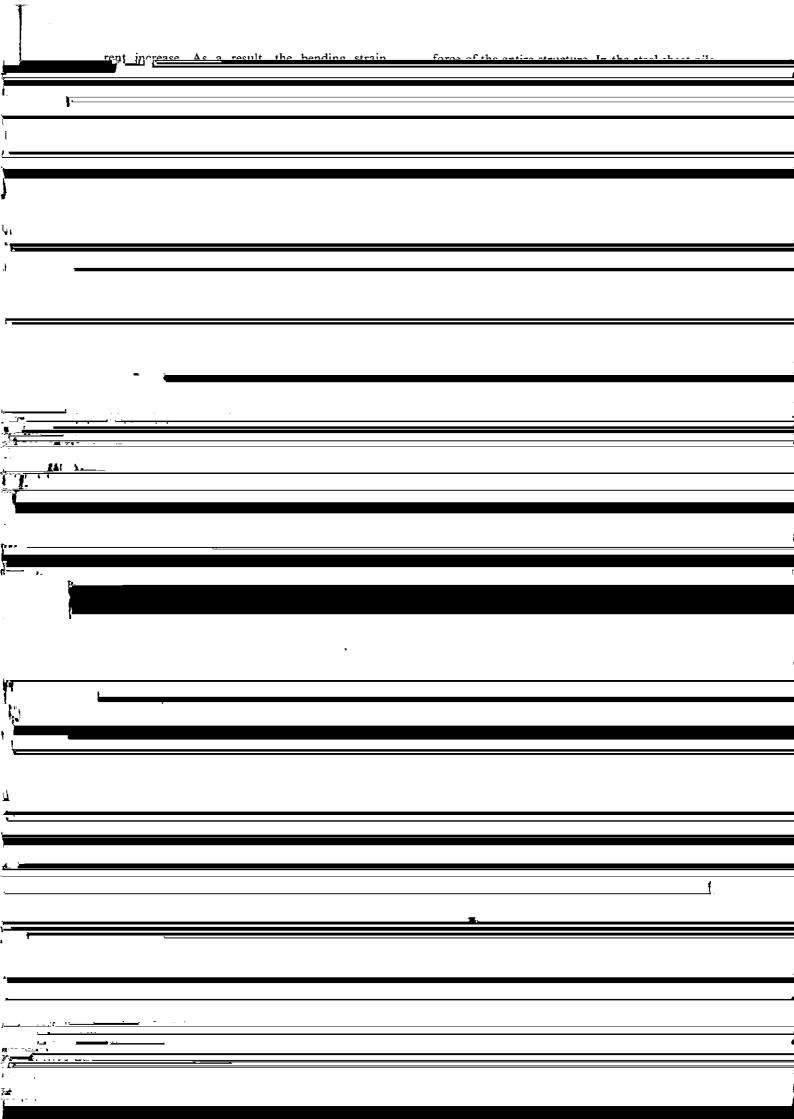




Photo 2 Typical failure mode of riveted "T" connec-

2.2.2 Failure modes

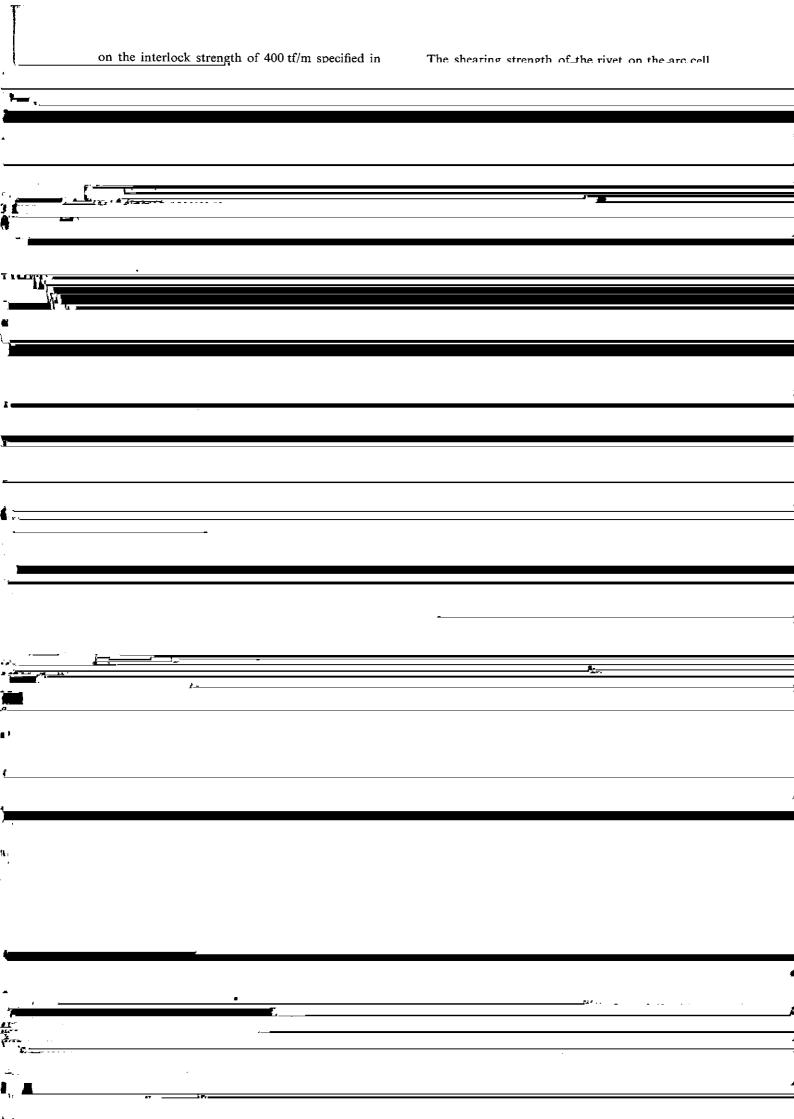
(1) Riveted connection type

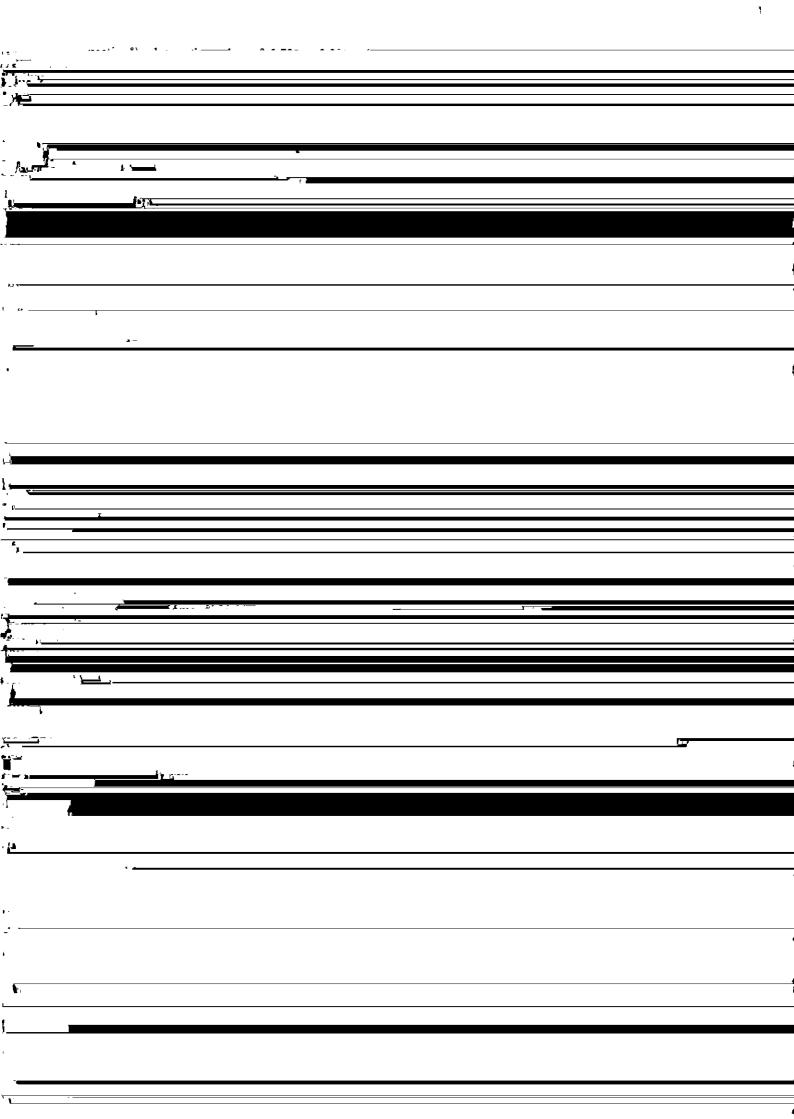
For the riveted "T" connection, ultimate strength is governed entirely by the shear strength of arccell-side-rivets. Photo 2 and 3 show the tested "T" connection sheet piles and its rivet after failure, respectively. In general when "T" connection sheet piles are designed to be rivet-connected, it is assumed that the circular-cell-side rivets are tensile rivets and arc-cell-side rivets are shearing or bearing ones, and the strength of the connections is checked by using a smaller value of the two types of rivets. Table 2 shows all types of allowable stress applicable to riveted connections of stress applicable to riveted connections of stress applicable to riveted connections of stress

tension



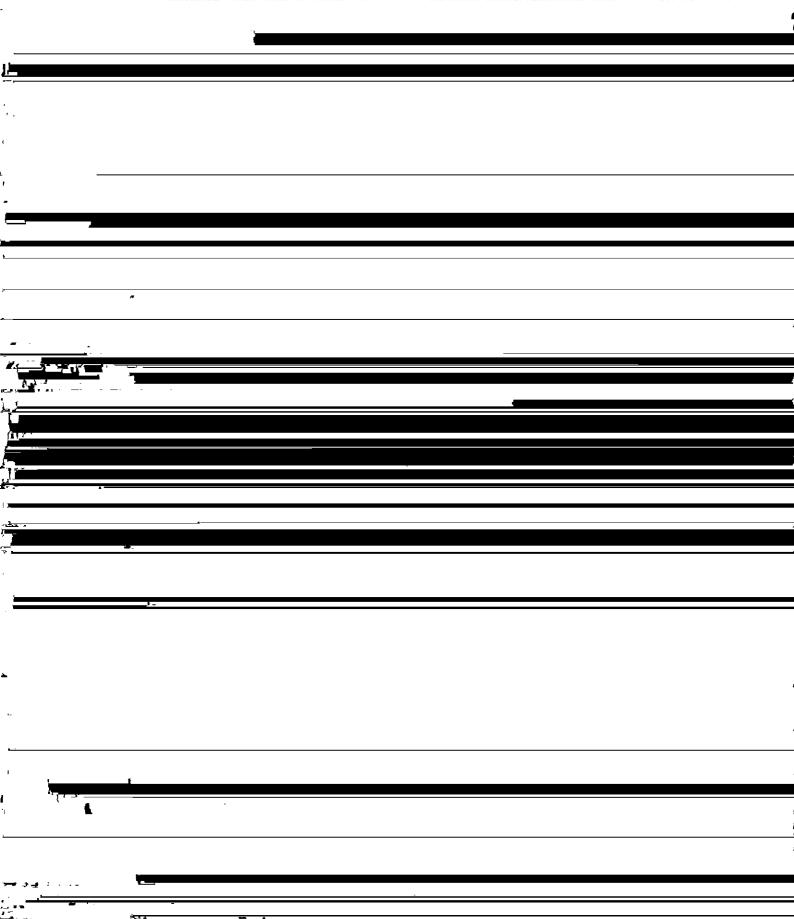
sheet piles. When these values are used in designing "T" connection sheet piles in order to obtain the guaranteed interlock strength of 400 tf/m, it is found that strength is insufficient. And no valid design based on the allowable stress will become possible, even if tensile rivets on the circular cell side are driven in the minimum pitch, on the assumption that the allowable tensile strength of straight web steel sheet pile is 150 tf/m and the





failure is, in ideal terms, governed by the tensile strength of the arc-cell-side sheet piles or shearing strength of the fillet weld. In general, previous studies⁹⁻¹¹⁾ reported that the bending deformability of weld was comparatively small. In the present test, the welded tee connection sheet piles reinforced with splice angles also generated a weld toe crack and fractured when load of about 50%

tf/m (both for cases having 4 bolts/m and 6 bolts/m) and of 150 tf/m (4 bolts/m) were subjected,—developed base-material-side fracture as shown in **Photo 6** (shear fracture starting from bolt hole). The bolts after this testing showed great shear deformation, thereby indicating that the combination of material strength values in the present test lay on the border separating the follows and Great

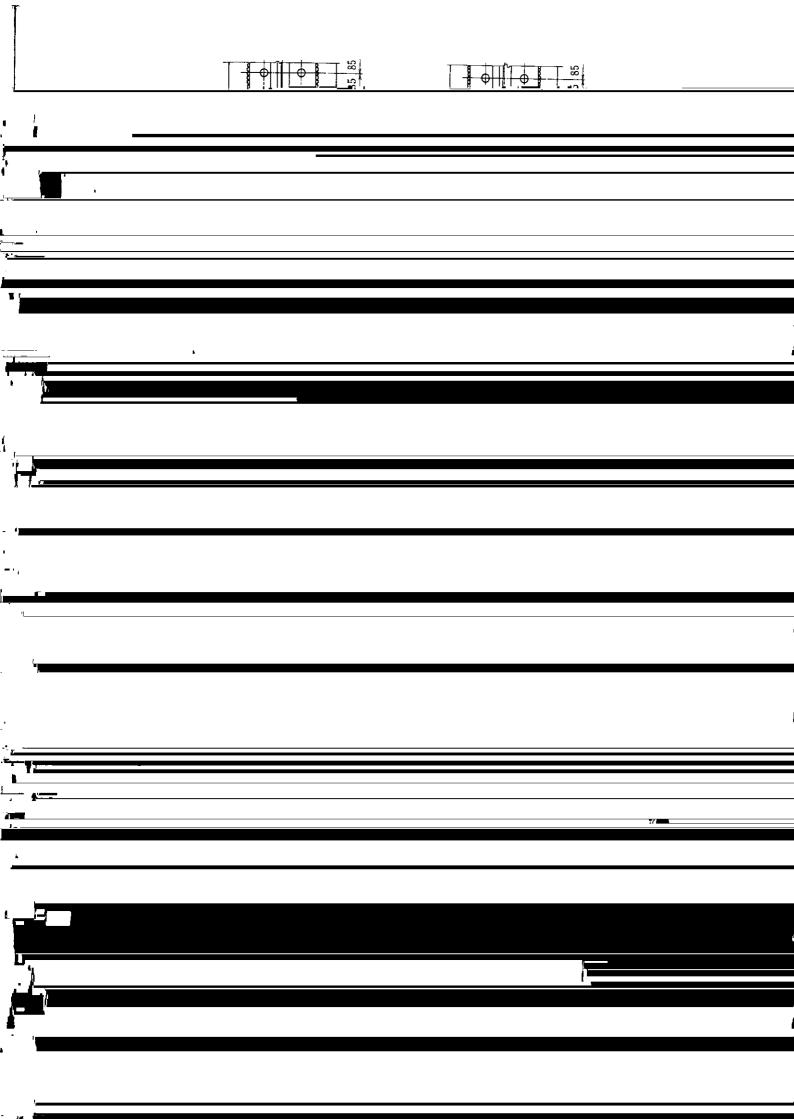


cates that high strength bolted tee connection is nection sheet piles, taking into consideration its special structural characteristics that bending is much more effective than the riveted (SV41) "T" unavoidably applied on the bolt head, its double connection for preventing decrease in ultimate choor strength T is given by the following equaetranoth caused by hole evolution on the circular cell side sheet piles and for reduction in the numtion: ber of rivets or bolts.

that $\alpha = 1/2$ in the high strength bolted tee con-

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be calculated as shown in Table 3. This table indi-



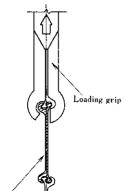
tension of 600 tf/m

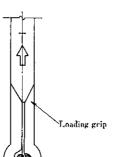
Material .		Yield strength (kgf/mm²)	Tensile strength (kgf/mm²)	Elongation
Sheet pile	SY30 t _W =12.7	37	58	23
FR	SM50A 1W+12.7	42.3	57.1	26
Splice plate	SM50A t=14 16	42.9	55.6	25
Rivet	SV41 φ 25	28	43	34

3.1.2 Test method

The present test was primarily aimed at developing "T" connection sheet piles for the high interlock strength straight web steel sheet piles which can ensure a minimum tensile strength in the arc-cell-direction of 400 tf/m. Therefore, no 2-direction simultaneous tensile test was performed, but separate 2 single-direction

tensile tests were performed by using separate tensile grips for tension in the arc-cell-direction and that in the circular-cell-direction as shown in Fig. 9. Dummy steel sheet piles were inserted between the tensile grip and "T" specimens which were made from the same lot as "T" sheet piles, so that failure at interlocks might occur when the connection strength of sheet piles is higher than the interlock strength.





bination type) which ought to have better weldability connection sheet piles for the high interlock straight shpws lower ultimate length than that of SY30 which web steel sheet piles can be obtained. has been considered inferior in weldability. This may be attributable to the following reason: For the 4 Conclusion specimen consisted of SY20 a standard of vicual inspection for weld configurations was set up and For the purpose of studying the ultimate strength ated in the design specification of the Japan Port 5) S. Kikukawa, K. Murata and A. Nishimura: "Secular Changes of Slip Resistance of Friction-type Bolted Joints in and Harbour Association33, the F-type riveted "T" Structural Members", Kawasaki Steel Technical Report, 11 connection type (rivet dia.: 25 mm, pitch: 85 mm) (1979) 4, pp. 127-135 (in Japanese) was proved to have sufficient ultimate strength of 6) T. Yagyu: Kowan, 57 (1980), 8, pp. 55-66