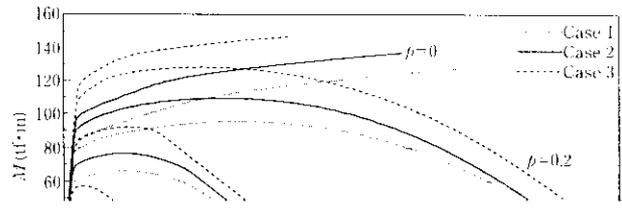
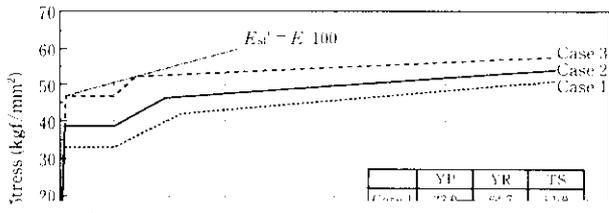


Structural Behavior of Super HISLEND-H*



Synopsis:

*The sufficient ductility of Super HISLEND-H for practical
construction was confirmed by an in-plane plate*



Strain (%)

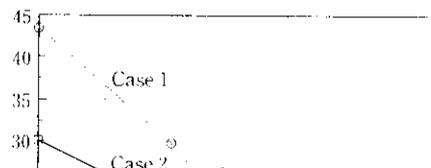
Fig. 1 Stress-strain curves for numerical analysis



Fig. 2 Loading condition

0.0 0.1 0.2 0.3 0.4 0.5 0.6 0.7 0.8
θ (rad)

Fig. 3 $M-\theta$ curves (SM490)



... of the ... test, therefore a specimen was used in-

... of the vertical stiffening plate was bent

The specimen was subjected to

loading, and the deformation and strain at various parts
of the specimen under loading were measured.

(1) C-B1: Tensile yielding of the column web → tensile

3.3 Test Results

Load (P)-deformation (Δ) curves of series B and T specimens are shown in Figs. 6 and 7, respectively. In

plane flexural yielding of the column flange → tensile yielding of the beam flange

(2) C-B2: Tensile yielding of the column flange → tensile flexural yielding of the vertical stiffening plate

where $t = t_r + t_c$

$t_r M_p$: Yielding moment per unit length of the



$$s_c P_p = 4 \sqrt{t M_p \cdot c b (s_1 T_y \cos \alpha_1 + s_2 T_y \cos \alpha_2 + w T_y) + (s_1 T_y \cos \alpha_1 + s_2 T_y \cos \alpha_2 + w T_y) t}$$

$$\dots \dots sh \geq 4 \sqrt{\frac{t M_p \cdot c b}{s_1 T_y \cos \alpha_1 + s_2 T_y \cos \alpha_2 + w T_y} + t}$$

$$s_c P_p = \frac{4 \sqrt{t M_p \cdot c b (s_1 T_y \cos \alpha_1 + s_2 T_y \cos \alpha_2) (sh - t)^2}}{\dots}$$

Table 2 General yield strength of B-series and T-series specimens

Specimen	Experimental $e P_p$ (tf)	Analytical $s_c P_p$ (tf)	$e P_p / s_c P_p$
C-B1	111.3	106.8	1.04
C-B2	106.7	97.0	1.10
C-B3	91.3	91.3	1.00
C-B4	74.2	72.7	1.02